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**New challenges for Urban Engineering Seismology**

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**DELIVERABLE**

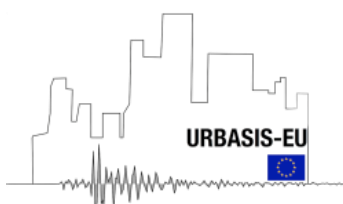
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**Work Package: WP2**

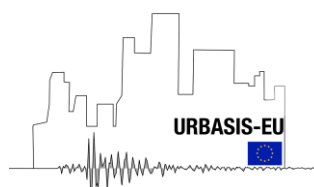
**Number: D2.3 – Development of Physical-Based Ground Motion Model for Induced Seismicity, Case Study: Preston New Road, UK**

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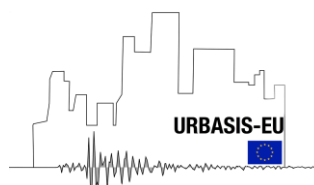
## 18 Abstract

19 The shale gas site at Preston New Road (PNR), near Blackpool, UK, has experienced shallow and  
20 small magnitude earthquakes causing some damage, observed mainly through cosmetic damage  
21 to structures in the surrounding areas. Quantifying risk assessment is important to reduce the  
22 possible threats of further, possibly larger, earthquakes. Better risk quantification will be  
23 influenced by a better determination of seismic hazard in the study area. Improvement of seismic  
24 hazard assessment through adapting and adjusting the available ground motion prediction  
25 equations (GMPEs) for near field- shallow earthquakes has been evaluated in this study,  
26 specifically for the PNR site. Stochastic simulation was performed for the development of an  
27 application-specific ground motion model for induced earthquakes in the PNR gas field by  
28 considering a new physically- based seismic attenuation model obtained from spectral fitting  
29 approach. The comparison between the new ground motion model (GMM) in the present study,  
30 with existing GMPEs from previous study, and GMPEs developed from other induced seismicity  
31 environments is presented. The comparison with the GMPE for tectonic seismicity provides  
32 explanation of the difference characteristic of ground motion model for near field- shallow  
33 earthquake and the deeper tectonic earthquake.

## 34 Introduction

35 Seismic events can produce injuries and fatalities, damage to buildings or infrastructure, in  
36 addition to interruptions to business and operation. To mitigate and reduce the potential threats of  
37 earthquakes, it is important to perform risk analysis and develop methodology and/or scenario to  
38 better manage earthquakes. The quantification of seismic risk requires quantification of seismic  
39 hazard as the input. Hazard assessment was carried out by studying local geology, tectonic settings,  
40 past historic earthquakes or seismicity of the area, and magnitude levels. This is necessary to  
41 estimate the intensity and ground shaking hazard that may occur in the various parts of the city.

42 The study and application of hazard estimation has evolved in recent decades through the  
43 identification of epistemic uncertainties (related with unknown knowledge) and introduction of a  
44 rational frameworks for handling the apparent randomness in earthquake processes (Bommer,  
45 2022). Despite the escalation of the hazard assessment studies, there are still several challenges  
46 found. One of the examples is the seismic hazard estimation for low-seismicity regions such as the



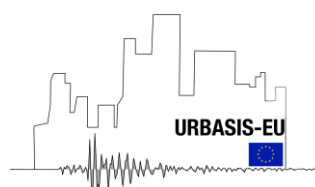
47 United Kingdom (UK). The lack of earthquake strong-motion records in the low-to moderate  
48 magnitude seismicity regions, such as the UK, means that trivial methods to estimate a GMPE for  
49 strong ground motion is no longer valid (Edwards et al.,2008). The selection of suitable existing  
50 GMPE and the adaptation for specific region become more profound in the case of anthropogenic  
51 earthquakes. This deliverable aims to improve our understanding of ground motions from induced  
52 seismicity in Blackpool, UK, and in particular, about the difference of ground motion  
53 characteristics between induced and UK regional seismicity. This comparison will help to better  
54 explain the bias observed (Edwards et al., 2021; Douglas et al.,2013) in the adapted regional  
55 GMPEs applied in the induced seismicity case.

56 The first section of this deliverable will cover a brief description about induced seismicity and  
57 its impact on seismic hazard analysis, and the state of the art of the ground motion models for  
58 induced seismicity applications. The overview of our datasets is explained in the second section.  
59 Finally, we review existing GMPEs for induced seismicity and present a new ground motion  
60 model, developed using physical-based stochastic simulations, for the PNR region.

## 61 Induced Seismicity

62 Induced seismicity, has become a more commonly discussed and pertinent topic for study in  
63 recent years due to a significant increase in the number of anthropogenic earthquakes. Historically,  
64 the first observation of induced seismicity was connected to mining activities in South Africa in  
65 early 1894 (Müller et al., 2021; McGarr et al, 2002). Davis & Frolich (1993), proposed a series of  
66 questions to classify as induced seismic or not. The term 'anthropogenic seismicity' can be  
67 considered where human activity is reasonably shown to be the cause, or at least a major influence,  
68 of earthquakes. Such anthropogenic earthquakes can be subdivided into 'triggered' and 'induced'  
69 events. Triggered events are predominantly of natural origin since the state of stress in the area is  
70 tending toward the condition of shear failure. In this case, human activity simply accelerates the  
71 fault's inevitable failure. Meanwhile, induced events are generated purely by human activity  
72 (Rietbrock et al., 2013).

73 According to 'The Human-Induced Earthquake Database' (HiQuake) there are 1239 projects  
74 reported to have generated induced earthquakes, with 33% dominated by fracking activities and  
75 25% (the second highest proportion) due to mining activities. This total number has increased by

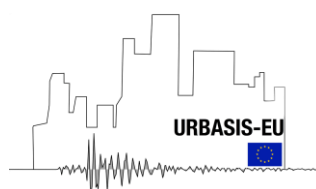


76 58.9% from last documentation by Wilson, et al. (2017), which reported a total ~730 projects  
77 associated with induced seismicity. In this case, the highest contributions were mining (37%) and  
78 the impoundment of water behind dams (23%). The injection activities and fracking account for  
79 ~10-15% of cases in this previous analysis. The fastest-growing anthropogenic activity is clearly  
80 induced earthquakes generated by fracking. This study will focus on ground motions that occur  
81 most-likely due to hydraulic fracturing. Hydraulic fracturing is commonly performed by drilling  
82 into tight-shale formations and injecting fluids under pressure to enable the production of oil and  
83 gas from previously unproductive formations (Ellsworth, 2013); The increase in pore pressure on  
84 a fault that can result from fluid injection reduces the effective normal stress acting on the fault  
85 and lowers the resistance to shearing, which can lead to a fault's rupture.

## 86 [Impact of Induced Seismicity on Seismic Hazard Analysis](#)

87 Despite of the small magnitude generated, induced seismicity can generate damage and become  
88 a concern. Shallower focal depths than tectonic seismicity, means shorter travel path from source  
89 to the surface, thus, generating higher ground motions than a deeper tectonic earthquake.  
90 Perceptible ground motion associated with industrial activities can cause distress to those who are  
91 affected, especially if there are many repeated episodes of shaking. This small magnitude event  
92 can cause public threat, damage to infrastructure, and affect interruption to business and operation  
93 and generate financial losses. Regardless of the understanding of the impact and possible risk due  
94 to induced seismicity, the implication of induced earthquake in the calculation of seismic hazard  
95 analysis remains unclear. One solution proposed by Walters, et al. (2015) is a site-specific and  
96 adaptable hazard and risk assessment and traffic-light protocol for injection projects.

97 The traffic-light system (TLS) is typically used to address the possibility of a variable seismic  
98 risk over time and allow for real-time risk management. Either the injection projects are  
99 recommended to continue (green), modify or re-evaluate due to increased risk (amber), or suspend  
100 operations due to severe risk (red). The fundamental purpose of a TLS is to avoid levels of ground  
101 shaking that would exceed tolerable limits, which would generally mean anything from causing  
102 damage to buildings in the vicinity of operation to causing a disturbance to the local populations.  
103 These site-specific protocols and assessments are useful for operations, but the question remains  
104 as to how the increasing rate and magnitudes of induced events will affect future assessments of

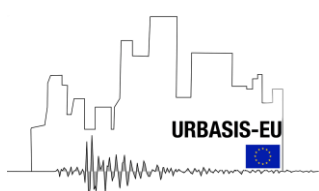


105 seismic hazard. Probabilistic seismic hazard assessment (PSHA) for induced seismicity can adapt  
106 PSHA studies of natural seismicity, where earthquake rates and ground motions are inferred from  
107 past observation or historical earthquakes catalogues. However, for induced seismicity, the  
108 equivalent observational metric is not the average numbers of earthquakes per year resulting from  
109 continuous long-term tectonic processes, but it is rather related to operational parameters such as  
110 pumping volume and pressure and the susceptibility of the subsurface to induced seismicity. The  
111 estimation of hazard for future operational scenarios is enhanced by relating the observations of  
112 induced earthquakes to a characteristic of the fluid injection, for example by using the seismogenic  
113 index (Shapiro et al., 2010), which relates the seismic activity rate to the total volume of injected  
114 fluid.

### 115 Ground Motion Prediction for Induced Seismicity

116 For many years, ground motion models (GMM) have been developed for application to tectonic  
117 earthquakes of magnitude 4.5 or greater. Extrapolation of such equations, derived from regression  
118 of data from larger magnitude earthquakes, has been shown to overestimate ground motions not  
119 only for smaller magnitudes, but even at the lower limit of the target magnitude range (Bommer,  
120 et al. 2007; Chiou, et al. 2010; Chiou and Youngs 2014; Douglas and Jousset 2011; Baltay and  
121 Hanks 2014). For induced seismicity, several GMPEs have been proposed, such as: (1) Dost et al.  
122 (2004) who developed GMPE derived from recordings of shallow induced earthquakes in the  
123 Netherlands; (2) Sharma et al. (2014) who recognised the need of application-specific GMPEs and  
124 derived predictive equation to estimates ground motion for induced seismic in The Geysers  
125 geothermal field in California; and (3) Douglas, et al. (2013) who derived empirical and stochastic  
126 equations using earthquake recordings resulting from shallow geothermal activity.

127 In many cases, ground motion models for induced seismicity were adapted from existing  
128 tectonic ground motion models. Atkinson & and Assatourians (2017) demonstrate the use of  
129 California tectonic earthquakes with depth between 2-6 km to approximate ground motion for  
130 induced seismicity in Central and Eastern North America (CENA) by introducing near-distance  
131 saturation for small-to-moderate earthquakes, and explicitly consider source parameters as a  
132 function of focal depth in the model's functional form. Atkinson (2015) developed a GMPE using



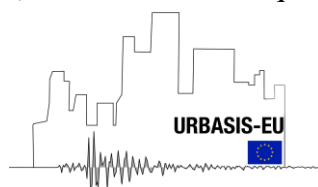
133 tectonic data, but limited to short distances and shallow depths, for use in induced seismicity  
134 settings.

135 An important question to consider is then the applicability of ground motion models, developed  
136 from tectonic earthquake, to estimate motions from induced events. Using a similar GMPE form  
137 to Atkinson (2015), Gupta et al. (2017) found that application of this model to induced seismicity  
138 in Central and Eastern United states (CEUS) results in a good fit for hypocentral distances up to  
139 60 km. Yenier & and Atkinson (2015) found that for the same tectonic setting and focal depth,  
140 ground motion for natural and induced earthquake appears to be similar. On the other hand,  
141 McNamara et al. (2019), while evaluating GMMs for USGS seismic hazard forecast: ‘Induced and  
142 Tectonic earthquakes in the Central and Eastern United States’, concluded that Next Generation  
143 Attenuation (NGA)-East GMMs and 2014 CEUS GMMs show better performance for CEUS  
144 tectonic earthquakes than induced earthquakes. The model proposed by Atkinson (2015) and  
145 Grazier (2017) score better for predicting CEUS induced earthquake ground motions (Farajpor,  
146 2021). However, it appears that these models overpredict ground motion in the distance range of  
147 10-40 km. Similar observation were found by Bommer, et al. (2016) for induced seismicity due to  
148 gas field compaction in Groningen, where the initial GMPE, borrowed from the neighbouring  
149 field, as developed by Dost et al. (2004), did not provide a good fit to the data.

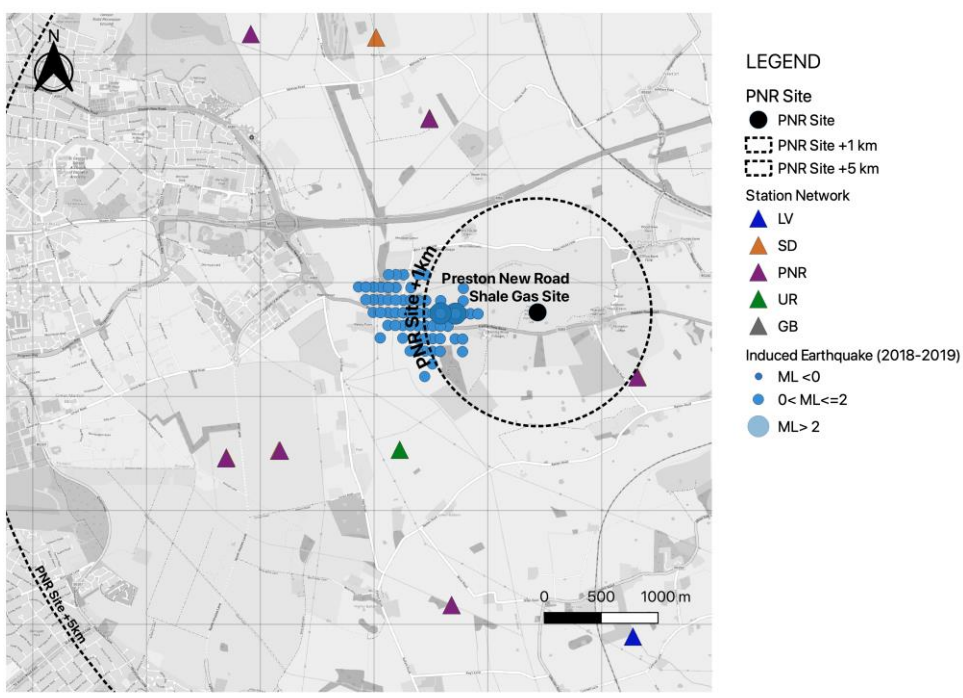
## 150 Data and Data Processing

151 A total of 192 events with 57 events ( $-0.8 \leq M_L \leq 1.5$ ) recorded in 2018 associated with  
152 hydraulic fracturing at PNR-1z and 135 events ( $-1.7 \leq M_L \leq 2.9$ ) recorded during the second  
153 phase at PNR-2 in 2019 were used for the induced seismic dataset in this study (Figure 1). These  
154 records were captured from several sensors from different networks (LV, PNR, SD, and UR)  
155 spread within 20 km from the PNR Shale Gas Site (Figure 2). Induced events were located at  
156 shallow depths of up to 4 km, with epicentral distances up to 20 km (Figure 3).

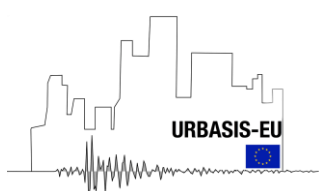
157 An additional dataset (network GB) from natural earthquake records was provided by the  
158 British Geology Survey (BGS). We henceforth refer to this as the tectonic dataset (Figure 4). This  
159 dataset consists of 308 events with magnitudes  $-1.4 \leq M_L \leq 4.2$  recorded in 2019. This dataset  
160 also contains small magnitude ( $M < 0$ ) and shallow earthquake identified as tectonic earthquakes



161 which was recorded near Charlwood, England, UK. This location close to the two actively  
 162 operating oilfield discovery and production sites at Brockham and Horse Hill (Hicks, et al., 2019).  
 163 These anomalous seismic swarms occurring at shallow depth can have natural causes (e.g., Bent  
 164 et al., 2017; Hicks et al., 2019). Catalogue of this dataset can be accessed through BGS website  
 165 (<https://www.earthquakes.bgs.ac.uk/>), while detailed information can be downloaded from  
 166 <ftp://seiswav.bgs.ac.uk/events> in Nordic file format. By joining event information and station  
 167 information, the complete catalogue was generated. Event waveforms were then retrieved from  
 168 the daily continuous ‘mseed’ files provided from the BGS repository. Most of the records  
 169 presented in this study come from the BGS network, due to the limited access of the data from  
 170 other networks (Figure 5). These records spread from 0 - 700 km, with focal depths up to 28 km  
 171 (Figure 6).

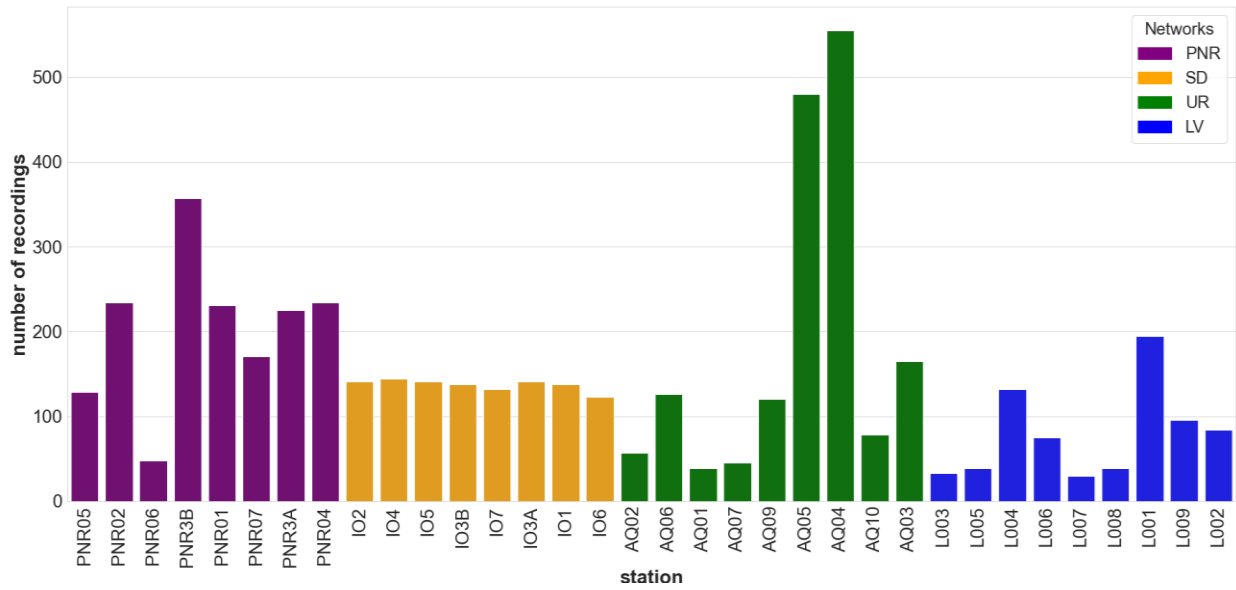


172  
 173 *Figure 1. Map of stations and induced event locations (recorded in 2018-2019 from Preston New Road site with local magnitude*  
 174 *range  $-1.7 \leq M_L < 3$ .*





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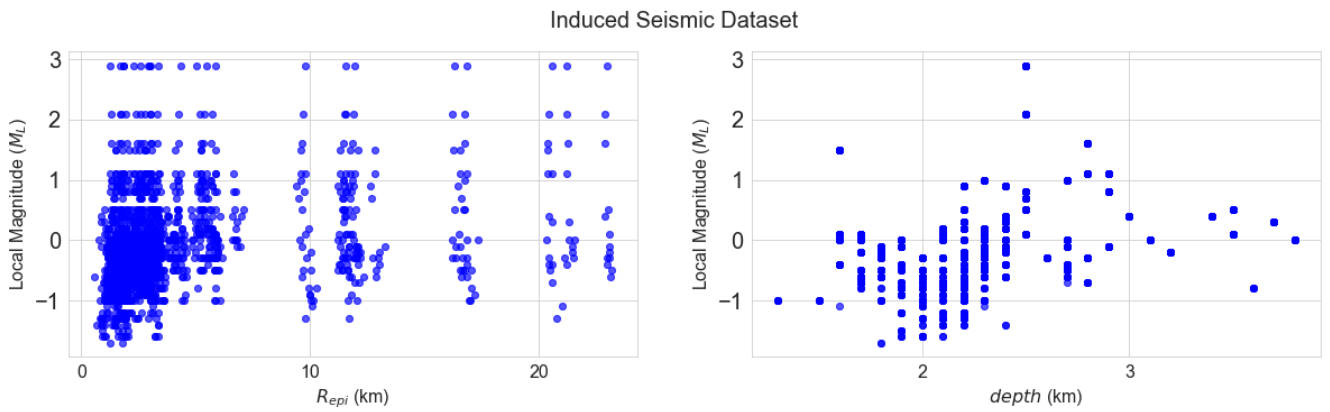


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177 *Figure 2. Number of recordings for each different stations for induced dataset. Colours indicate each different network.*

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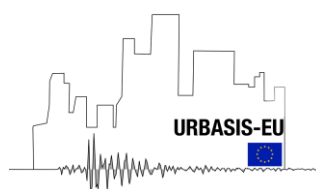
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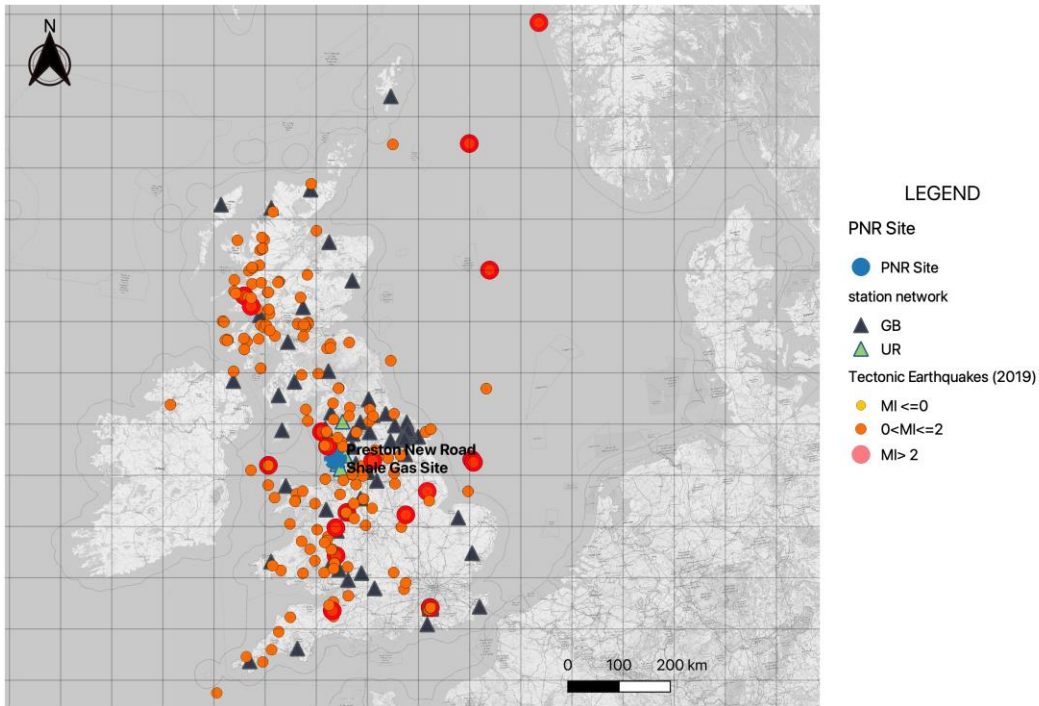


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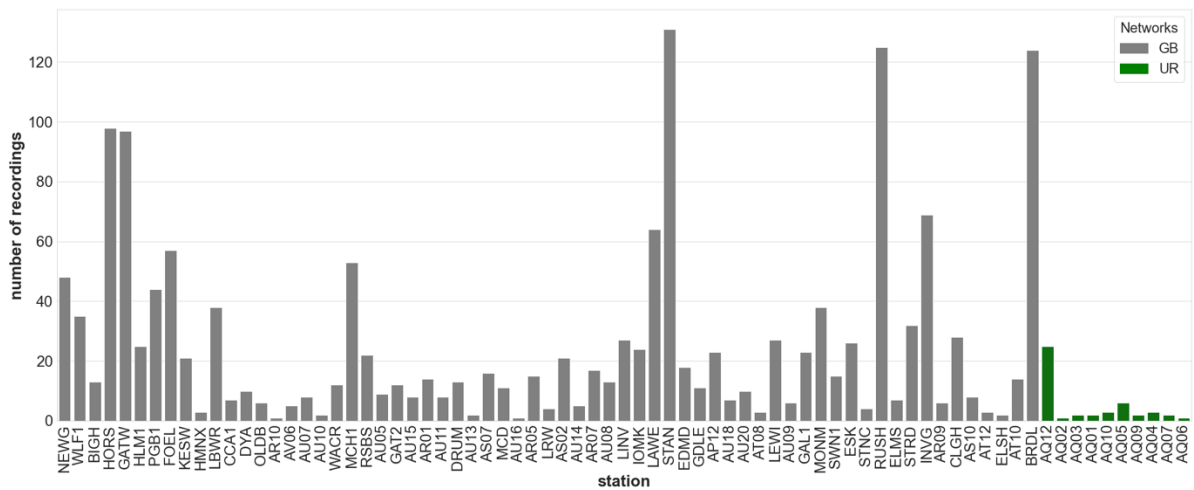
181 *Figure 3. Earthquakes magnitude as function of distance (left) and depth (right) for induced dataset.*

182



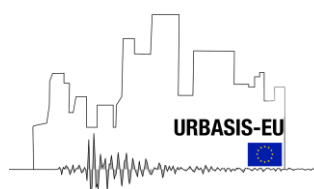


183  
 184 Figure 4. Map of stations and tectonic event locations (recorded in 2019) with local magnitude range  $-1.4 \leq M_L < 4.6$  (source:  
 185 BGS website, last accessed: June 2022).

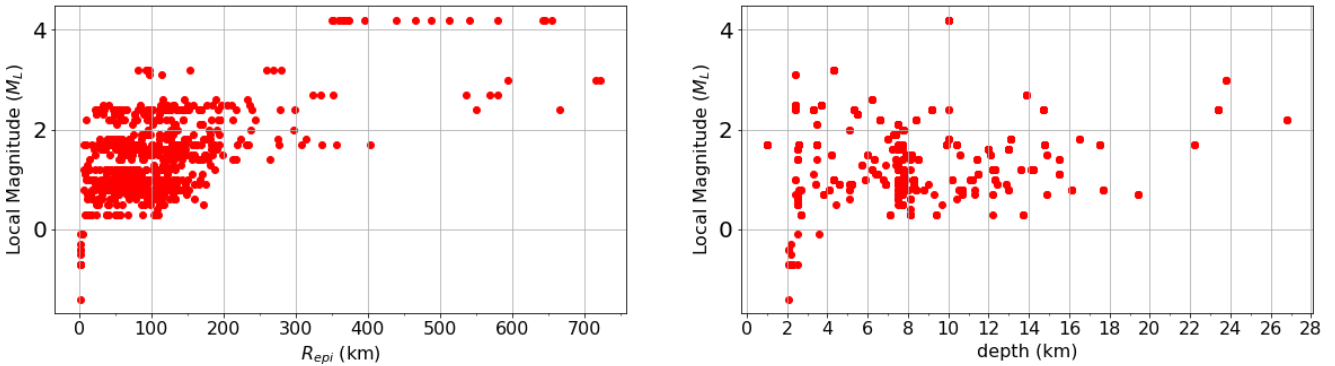


186  
 187 Figure 5. Number of recordings for each different stations for tectonic dataset. Colours indicate each different network.

188



### Tectonic Dataset- 2019



189

190

Figure 6. Earthquakes magnitude as function of distance (left) and depth (right) for tectonic dataset.

191

The original seismic records need to be pre-processed in order to calculate the observed peak ground acceleration (PGA) or peak ground velocity (PGV). Several steps are performed, including: (1) signal detrending, (2) removing the seismic instrument response, (3) recalculating the end of the signal, which corresponds with 95% of the cumulative energy; and (4) finding the highest amplitude value. The maximum absolute value of the amplitude on respective traces is used to calculate record-specific PGA and PGV.

196

## 197 Ground Motion Prediction Equation for Induced Seismicity, Case Study at 198 Preston New Road, UK

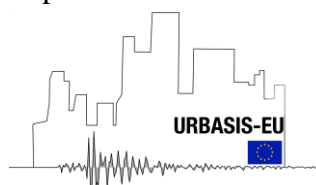
### 199 Existing GMPE for Induced Seismicity at Preston New Road, UK

200

201

The performance analysis of the available ground motion models has been assessed at the PNR site by Edwards et al. (2021). The recorded ground motion has been compared with the prediction estimated using Atkinson (2015) and Douglas et al. (2013) models. The comparison of these involved residual analysis of both models, aiming to select most suitable model for subsequent adjustment using the referenced empirical approach (Bommer, et al., 2006; Atkinson, 2008). The first model tested was the GMPE of Douglas et al. (2013), developed specifically for geothermal induced seismicity, with data consisting of events with magnitude  $M \geq 1$  and  $R < 30 km$ . According to Edwards et al. (2021), the ground motion overall is overpredicted. The overprediction tended to increase with the decrease of magnitude. These bias presumably due to significant regional differences. It is important to consider the site- specific or non-linearity effect,

210



211 which is not addressed in the model proposed by Douglas et al. (2013). The significant misfit to  
 212 local data and limited flexibility of the model for calibration to that data (for example, including  
 213 only linear magnitude scaling) meant that this model is not selected for application in PNR site.

214 The next model tested was the Atkinson (2015) GMPE model (A15), developed based on the  
 215 NGA-West2 dataset which consists of  $M$  3 - 6 earthquakes. The majority of smaller earthquake  
 216 are corresponding with Californian tectonic earthquakes which are not necessarily shallow. These  
 217 records were limited up to 40 km hypocentral distance to focus on the near-source motion, typical  
 218 of focus for induced seismicity. The A15 model is made for a rock reference site with  $V_{s30} = 760$   
 219 m/s. To adjust to local conditions at PNR, which has significantly lower  $V_{s30}$ , the site response  
 220 model of Boore et al. (2014) was used ( $V_{s30} \sim 200 - 300$  m/s). The A15 GMPE model can be written  
 221 as:

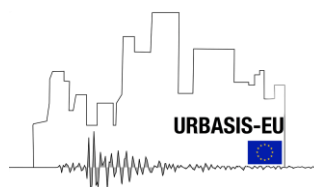
$$X = \Delta c_0 + \Delta c_1 M + \Delta c_2 M^2 + \Delta c_3 \log_{10} R + B_e + W_S \quad (1)$$

223 with  $X$  is the ( $\log_{10}$ ) peak acceleration for a given spectral ordinate,  $c_i$  are the coefficients,  $M$  is  
 224 the moment magnitude and  $R$  is an effective distance, as defined in Atkinson (2015):

$$R = \sqrt{R_{hyp}^2 + \max(1, 10^{-0.28+0.19 M})^2} \quad (2)$$

227  $B_e$  in the equation 1 are the random effects for the events, and  $W_S$  are the station-specific random  
 228 effects.

229 Based on the residual analysis by Edwards et al. (2021), a good fit is found at distances  $R_{epi} >$   
 230 5 km, but the model significantly underestimates at shorter distances. To minimise the sigma of  
 231 prediction, calibration of the A15 model was therefore undertaken. This effort is in line with  
 232 Bommer et al. (2006) and Atkinson (2008), which shows the improvements in the prediction of



233 local ground motion estimates by using the referenced empirical approach. Edwards et al. (2021)  
 234 perform the calibration using a mixed-effects regression. The adjustment, constrained by the data  
 235 at available magnitude and distances, can be expressed as:

236

$$\begin{aligned}
 \log_{10} Y_{E20} &= d_0 + d_1 \mathbf{M} + d_2 \mathbf{M}^2 + d_3 \log_{10} R && \text{for } \mathbf{M} < 3 \\
 \log_{10} Y_{E20} &= p_0 + p_1 \mathbf{M} + p_2 \mathbf{M}^2 + p_3 \log_{10} R && \text{for } 3 \leq \mathbf{M} < 4.5 \\
 \log_{10} Y_{E20} = Y_{A15} &= c_0 + c_1 \mathbf{M} + c_2 \mathbf{M}^2 + \Delta c_3 \log_{10} R && \text{for } \mathbf{M} \geq 4.5
 \end{aligned} \tag{3}$$

237

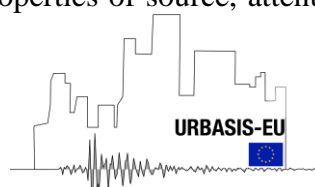
238 where  $d_i$  are the calibrated coefficients (Edwards et al., 2021), and  $p_i$  are linearly interpolated  
 239 between original A15 coefficient ( $c_i$ ) and calibrated coefficient ( $d_i$ ), which can be written as:

$$p_i = c_i + \frac{\mathbf{M} - 4.5}{3 - 4.5} \Delta c_i \tag{4}$$

240 The difference is higher near-field short period motions in the calibrated A15 model, presented  
 241 by Edwards et al. (2021), compared to original Atkinson (2015) model. The application of both  
 242 models will be presented in the last chapter of this report (general discussion and conclusion).

### 243 Stochastic Ground Motion Model for Preston New Road, UK

244 Adapting and adjusting existing GMPEs may lead to predictions that are not robust and or  
 245 potentially biased. One of the limitations of GMPE adjustment using the hybrid empirical method  
 246 is that they must be converted into equivalent Fourier models by calculating minimum misfit  
 247 Fourier Acceleration Spectrum (FAS) based models (Campbell, 2003; Scherbaum et al., 2006) or  
 248 response spectra consistent FAS (Atik et al., 2014). Any physically based adjustment is inherently  
 249 complex due to the simplified basis of empirical response spectra-based models. An alternative  
 250 approach is by directly use FAS model based and shaking-duration, which related through  
 251 stochastic simulation (Boore, 2003; Motazedian & Atkinson, 2005) or random-vibration theory  
 252 (RVT) (Atkinson & Boore, 2006; Edwards & Fäh, 2013; Drouet & Cotton, 2015). Such models  
 253 are calibrated based on physical properties of source, attenuation, and site conditions which are



254 modelled or measured from earthquake recordings (e.g., Edwards et al., 2008; Drouet et al., 2011;  
 255 Bommer, et al., 2016). The advantage of FAS based models (e.g., stochastic simulation model) are  
 256 easier to adjust than the empirical GMPEs (Bora et al., 2013) and can easily linked to physical  
 257 processes (Baltay et al., 2017). Besides, the epistemic uncertainty may also be easier to quantify:  
 258 the physically interpretable parametric variations such as stress drop, attenuation, and site  
 259 amplification can be specified as distributions rather than unique values. Taking the advantage of  
 260 such stochastic simulation approaches, this study focuses on the development of physical-based  
 261 ground motion model for induced seismicity.

262 The stochastic method proposed by Boore (2003) is a simple and powerful tool (so called  
 263 Stochastic- Method SIMulation or SMSIM) to simulate ground motion by combining parametric  
 264 or functional form of the ground motion’s amplitude spectrum with a random phase spectrum. The  
 265 stochastic method of Boore (2003) is used to simulate ground motion for the Preston New Road  
 266 (PNR) site in terms of PGV and PGA. This method is relying on the knowledge of the expected  
 267 Fourier spectrum of an earthquake recording with a given magnitude and distance. The spectrum  
 268 of earthquake ground motion  $i$  recorded at station  $j$  that has been corrected with instrument  
 269 response can be described as:

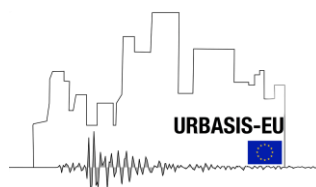
$$\Omega_{ij}(f) = E_i(f) \times P_{ij}(f) \times S_j(f) \quad (5)$$

270 where  $f$  is the frequency, and  $E_i(f)$  is the amplitude spectrum of “Brune” source model,  $P_{ij}(f)$  is  
 271 the attenuation along the ray path, and  $S_j(f)$  is the site term. By separating the spectrum into  
 272 source, path, and site components, the models can be easily modified to account for specific  
 273 situations.

274 The shape and the amplitude of the source spectrum should be specified as a function of  
 275 earthquake size. The source model can be written as:

$$E_i(f) = \frac{\Omega_0}{1 + (f/f_c)^2} \quad (6)$$

276



277 The  $\Omega_0$  term contains seismic moment and other frequency-independent effects. The seismic  
 278 moment ( $M_0$ ) of a seismic record expressed as:  
 279

$$M_0 = \frac{4 \pi \rho v^3 R_{hyp} \Omega_0}{F_s R_{\theta\phi}} \quad (7)$$

280 where  $\rho$  is the rock density at the source ( $2800 \text{ kg/m}^3$ ),  $v$  is the velocity in the source ( $v =$   
 281  $2000 \text{ m/s}$ ),  $R_{hyp}$  is the hypocentral distance, and  $\Omega_0$  is the low-frequency plateau.  $F_s$  is the free  
 282 surface amplification factor ( $F_s = 2$  for normally incident SH waves and a good approximation  
 283 for SV) and  $R_{\theta\phi}$  is the average radiation pattern coefficient for S-waves (0.55) (Boore &  
 284 Boatwright, 1984).

285  
 286 In the stochastic simulation approach, we used moment magnitude rather than seismic moment  
 287 as a more familiar measure of earthquake size. The relation between seismic moment and moment  
 288 magnitude is:

$$M_W = \frac{2}{3} \log M_0 - 6.03 \quad (8)$$

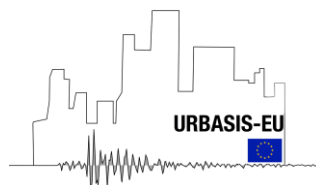
289 Since the earthquake sizes in the data catalogue were defined in terms of local magnitude ( $M_L$ ),  
 290 this value was converted to moment magnitude ( $M_W$ ) by following the  $M_L - M_W$  relationship  
 291 explained in Edwards et al. (2021). For the smallest events:  
 292

$$M_W = \frac{2}{3} M_L + 0.833 \quad (M_L < 1.5) \quad (9)$$

293  
 294 and for larger events, the  $M_L - M_W$  relationship model from Grünthal et al. (2009)

$$M_W = 0.0376 M_L^2 + 0.646 M_L + 0.53 \quad (M_L \geq 2.5) \quad (10)$$

295 with linear interpolation between both equation 9 and 10.



296 The path effect is representing the effect of geometrical spreading attenuation, and duration.  
297 The latest version (August 2021) of SMSIM allows the power of frequency in the  $Q$  model,  
298 therefore the attenuation along the ray path [ $P_{ij}(f)$ ] can be written as:

$$P_{ij}(f) = e^{-(\pi f^{1-\alpha} t^*)} \quad (11)$$

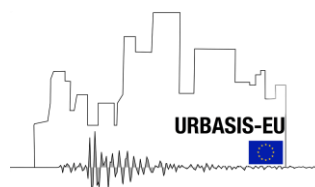
299 where  $t^*$  is the attenuation parameter ( $t^* = T/Q_0$ , with  $T$  the travel time and  $Q_0$  as the path-  
300 average quality factor at the reference frequency, here 1 Hz).  $\alpha$  describes the frequency  
301 dependence of  $Q$ , with  $Q(f) = Q_0 f^\alpha$ .

302 The site effect  $S_{ij}(f)$  is usually controlled by the amplification function [ $A(f)$ ] and diminution  
303 or damping function [ $D(f)$ ]. This generally represented by a high frequency decay function or  
304 kappa, an exponential function to explain the additional high frequency attenuation as a  
305 characteristic of local site attenuation (Anderson & Hough, 1984).

#### 306 Model Parameters

307 The ground motion can be simulated in two different ways: time-domain simulation and  
308 estimation using random vibration theory. In the time domain simulation, a time series envelope  
309 of Gaussian noise (with defined duration) is convolved with the target spectrum in the frequency  
310 domain. Returning to the time domain provides the simulated accelerogram. In practice, due to the  
311 random nature of the simulated time series, and the fact that we only need the peak amplitudes,  
312 random vibration theory is implemented to achieve the same result faster in this study. To perform  
313 the stochastic simulation, several parameters are defined. The attenuation model and kappa utilised  
314 in this study were defined from inversion scheme in the spectral fitting method detailed in Edwards  
315 et al. (2008) and (Suroyo & Edwards, 2023).

316 The spectral inversion scheme was performed for each individual record, following the steps  
317 presented in Figure 7, to fit  $t^*$ ,  $f_c$ ,  $\Omega_0$ , and  $\alpha$  (with a grid-search in the range 0-1), which allows  
318 us to calculate both frequency dependent and frequency independent  $Q$  measurements. This fitting  
319 approach was performed under the assumption that a single event from different recordings will

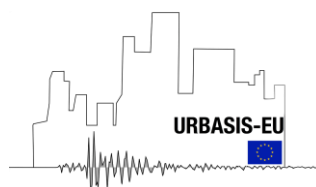


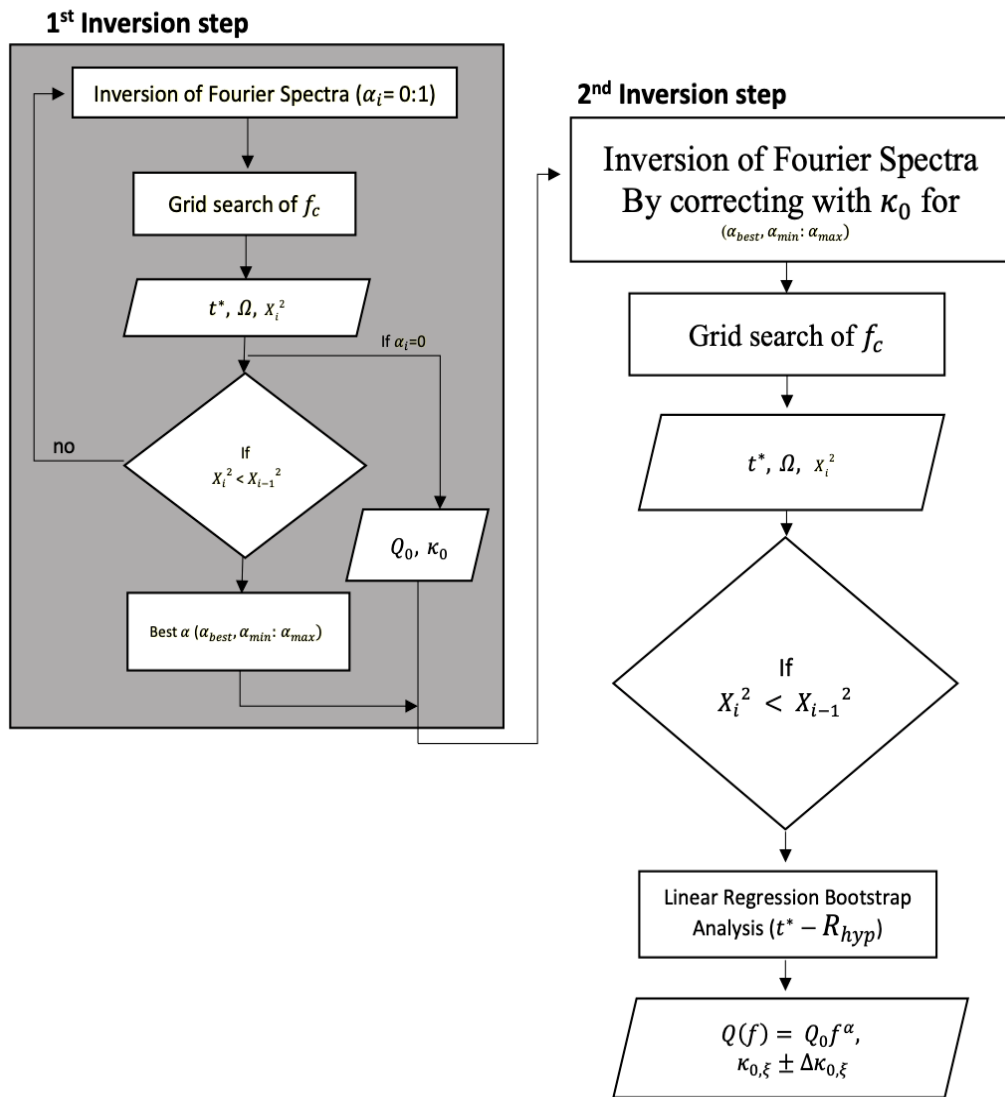


320 share same seismic moment and Brune stress drop and are therefore fitted to an event-specific  
321 corner frequency ( $f_c$ ). In Suroyo & Edwards (2023), the inversion scheme followed two steps:

322 (1) The first step of the inversion aimed to find the optimum value for  $\alpha$  (frequency  
323 dependence of  $Q_0$ ) by calculating the minimum chi-squared ( $X_i^2$ ) misfit (later denoted as  
324  $\alpha_{min}$ ) and to calculate the frequency-independent  $Q_0$  and  $\kappa_0$ .  $\alpha_{min}$  was estimated by  
325 calculating the chi-squared ( $X_i^2$ ) misfit over the log spectral amplitudes for  $\alpha$  within the  
326 range 0.0 - 1.0. The model misfit over the ensemble of observations for each given  $\alpha$  can  
327 be quantified. In this step, a frequency-independent model ( $Q_0$  and  $\kappa_0$ ) was also determined  
328 using linear regression of  $t^*$  versus  $R_{hyp}$  for a given  $\alpha = 0$ .

329 (2) The second inversion step was performed by fixing various elements based on the  
330 previous inversion results. A correction for frequency-independent  $\kappa_0$  is applied and the  
331 final inversion performed using a grid search over  $f_c$  between 0 – 50 Hz, fixing discrete  
332 selections of  $\alpha_{min}$  (lower limit of  $\alpha_{min}$  : upper limit of  $\alpha_{min}$ ). The product of the second  
333 inversion is then the final  $Q(f)$  and frequency-dependent  $\kappa_{0,\zeta}$  model (i.e.,  $Q_0$ ,  $\alpha$ ,  $\kappa_0$ , and  
334  $\kappa_{0,\zeta}$ ).

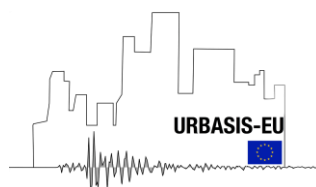




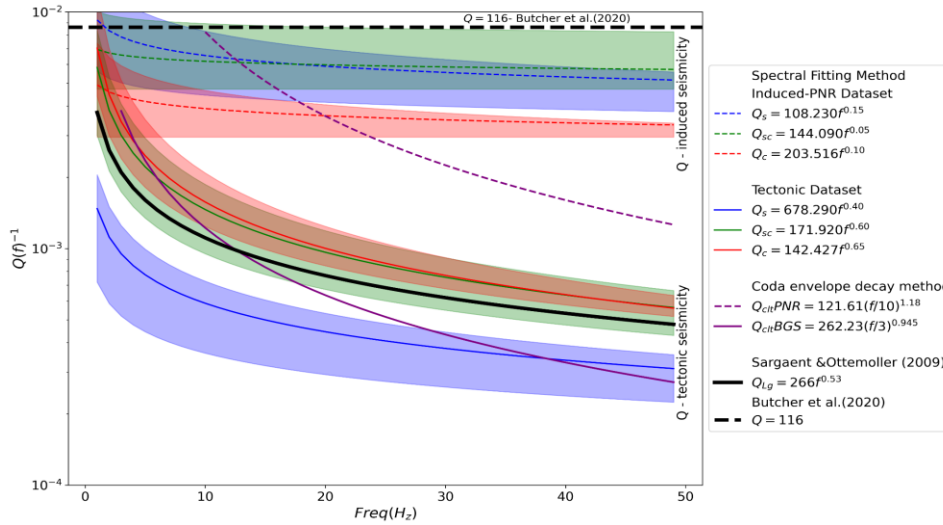
335  
336 Figure 7. Flowchart of inversion process for the spectral fitting method.

337

338 The local  $Q(f)$  obtained from the induced seismic sequences at Preston New Road (PNR) shale  
 339 gas site, attributed to shallower layers in the crust, leads to a rapid rate of near-field decay (sudden  
 340 loss in amplitude of earthquake signal over a short distances), with significantly stronger  
 341 attenuation than observed for regional events. We furthermore find that estimates of seismic  
 342 attenuation quality factor ( $Q_0$ ) are non-unique to a given record, differing both with the method  
 343 and the analysis windows used, particularly at high frequency (Figure 8). The lower overall  $Q(f)$   
 344 (stronger attenuation) in the induced seismicity records and the decreasing efficiency of scattering



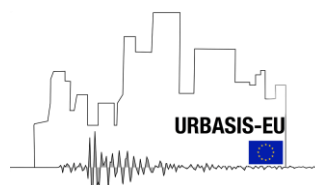
345 effects at short-scale distances, justifies that directly adapting tectonic GMPE for induced  
 346 seismicity will produce bias. Therefore, to predict ground motion models for shallow earthquakes,  
 347 it is important to consider the rapid rate of attenuation observed at very near-distances. Besides the  
 348 attenuation model, the site-specific high-frequency decay ( $\kappa_0$ ) were obtained as a side product of  
 349 the spectral fitting approach (Table 1). For stochastic simulation, we utilised the frequency-  
 350 dependent  $Q$  model (result from stage 2 inversion scheme) and the classic or frequency-  
 351 independent  $\kappa_0$  (from stage 1 of inversion scheme).



352  
 353 *Figure 8. Attenuation model from spectral fitting and coda envelope decay method for induced and tectonic dataset. The shaded*  
 354 *colour shows the confidence interval of the  $Q$  model (Suroyo & Edwards, 2023).*

355  
 356 *Table 1.  $Q_0$  (1 Hz),  $\alpha$ ,  $\kappa_{0,\zeta}$ (1Hz) and  $\kappa_0$  from spectral fitting method for both induced and tectonic dataset (results for frequency-*  
 357 *dependent and frequency-independent models from S wave windows).*

	Frequency-dependent model Final Model After Corrected with $\kappa_0$ (second inversion)				Frequency-independent model (initial inversion; $\alpha = 0$ )	
	$\alpha \pm \Delta\alpha$	$Q_0$ -Interval	$Q(f) = Q_0 f^\alpha$	$\kappa_{0,\zeta} \pm \Delta\kappa_{0,\zeta}(s)$	$Q_0$	$\kappa_0 \pm \Delta\kappa_0(s)$
<b>Induced</b>	$0.15 \pm 0.1$	[67.8- 178.6]	$108.2 f^{0.15}$	$0.033 \pm 0.0013$	169.6	$0.018 \pm 0.001$



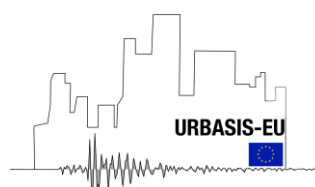
<b>Tectonic</b>	$0.4 \pm 0.1$	[487.8- 1387.9]	$678.3 f^{0.4}$	$0.06 \pm 0.012$	2953.9	$0.029 \pm 0.003$
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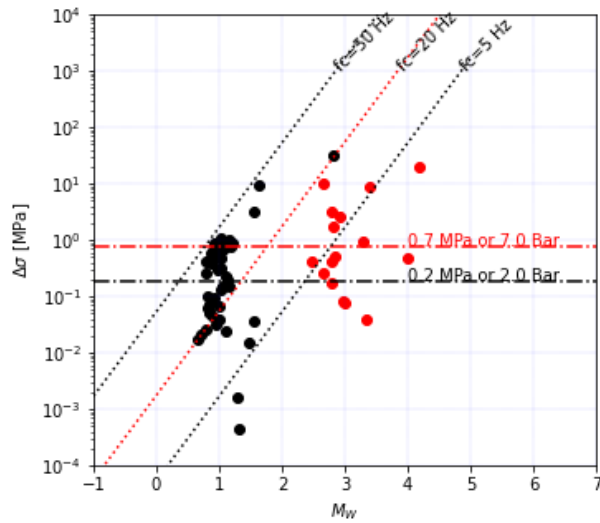
358

359 In addition to the attenuation model, stress drop was estimated using:

$$\Delta\sigma = \frac{7}{16} \left( \frac{M_0}{(0.372 * V_s / f_c)^3} \right) \quad (12)$$

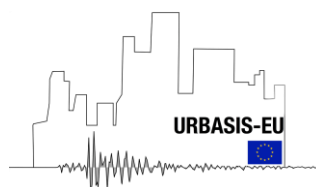
360 Stress drop describes the difference in shear stress on a fault before and after an earthquake,  
361 which can have a strong influence on ground motions for frequencies of engineering concern  
362 (Hanks, 1979; and Boore, 1983). While Huang et al. (2017) suggests that ground motion  
363 prediction equations developed for tectonic earthquakes can be applied to induced earthquakes  
364 (after properly considering the effects of depth and faulting style), in some cases, stress drop of  
365 induced earthquakes is not comparable with the stress drop from tectonic earthquakes. Hough  
366 (2014) infers that induced earthquakes have lower stress drops than tectonic earthquakes based on  
367 a comparison of non-instrumental “Did You Feel It?” intensities. In this study, we take the power  
368 10 of log average stress drop from each different dataset (tectonic and induced seismicity) (Figure  
369 9). The stochastic simulation is then performed using stress drop equal to 7.0 Bar for tectonic  
370 seismic and 2 Bar for induced seismicity, with other parameters detailed in Table 2.



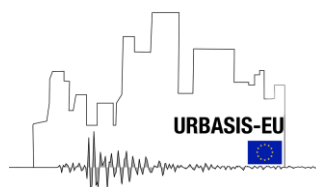


371  
 372 Figure 9. Stress drops vs moment magnitude for induced dataset (black) and tectonic dataset (red) from spectral fitting approach.  
 373 Table 2. Summary of input parameters for SMSIM in this study.

Parameter	Model	Description
Rho (gm/cc)	2.8	Density of the medium
Beta (km/s)	2.7 for induced dataset and 3.5 for tectonic dataset	Velocity of the medium
Partition factor (prttn)	0.71	Partition factor
Radiation pattern	0.55	Radiation pattern
fs	2.0	Free surface factor
Stress specification	stress = stressc*10.0**(dlsdm*(amag-amagc)), where stressc = 100.0, dlsdm = 0.0, amagc = 7.0, fbdfa=4.0	Parameters control the scaling of spectral amplitudes with source size.  Stressc: stress dlsdm: derivative of log sigma with respect to magnitude amagc: critical magnitude fbdfa: corner frequency fb divided by fa
Geometrical spreading	r_ref = 1.0 km, nsegs = 3 r_low (1) = 1.0, a_s = -1.0, b_s = 0.0, m_s (1) = 6.5 r_low (2) = 70.0, a_s = 0.0, b_s = 0.0, m_s (2) = 6.5	Parameters control the geometrical spreading.  R_ref: reference distance Nsegs: number of segments, each segment starting at r_low. a_s, b_s, and m_s are the coefficients of the slope of linen segment, can be written as: $slope(j) = a_s(j) + b_s(j)(M - m_s(j))$



	r <sub>low</sub> (3) = 130.0, a <sub>s</sub> = -0.5, b <sub>s</sub> = 0.0, m <sub>s</sub> (3) =6.5	
Anelastic attenuation model (Q)	fr1=0.1, Qr1=108.2 (for induced), 678.3 (for tectonic) s1=0.15(for induced), 0.4 (for tectonic) ft1=1.0, ft2=1.0, fr2=1.0, qr2= 108.2 (for induced), 678.3 (for tectonic) s2=0.15(for induced), 0.4 (for tectonic) c <sub>q</sub> =2.7(for induced), 3.5 (for tectonic)	Q(f) is given by a piecewise continuous set of three straight lines in log Q and log f space. The first and third lines have slopes of s1 and s2 and values of Qr1 and Qr2 at reference frequencies fr1 and fr2, respectively.  ft1, ft2 are the transition frequencies. c <sub>q</sub> is the velocity used in deriving the Q function.
Source duration	0.5 (weights of 1/fa, 1/fb)	Source duration
Path duration	nknots = 4 rdur (1) = 0.0, dur (1) = 0.0 rdur (2) = 10.0, dur (2) = 0.0 rdur (3) = 70.0, dur (3) = 9.6 rdur (4) = 130.0, dur (4) = 7.8 slope of last segment =0.04	specification of the path duration nknots is the number of intersections between line segments.
Crustal amplification	Namps= 5 famp (1) = 0.1, amp (1) = 1.0 famp (2) = 1.0, amp (2) = 1.5 famp (3) = 2.0, amp (3) = 2.0 famp (4) = 5.0, amp (4) = 2.5 famp (5) = 10, amp (5) = 3.0	Site amplification is approximated by a series of straight-line segments in log amplification, log frequency space, connecting the values famp, amp.  Namps is the number of segments.
Site diminution parameters	fmax=50.0, kappa= 0.018(for induced), 0.029 (for tectonic)	Site diminution factor

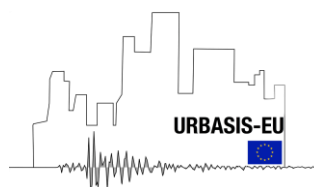


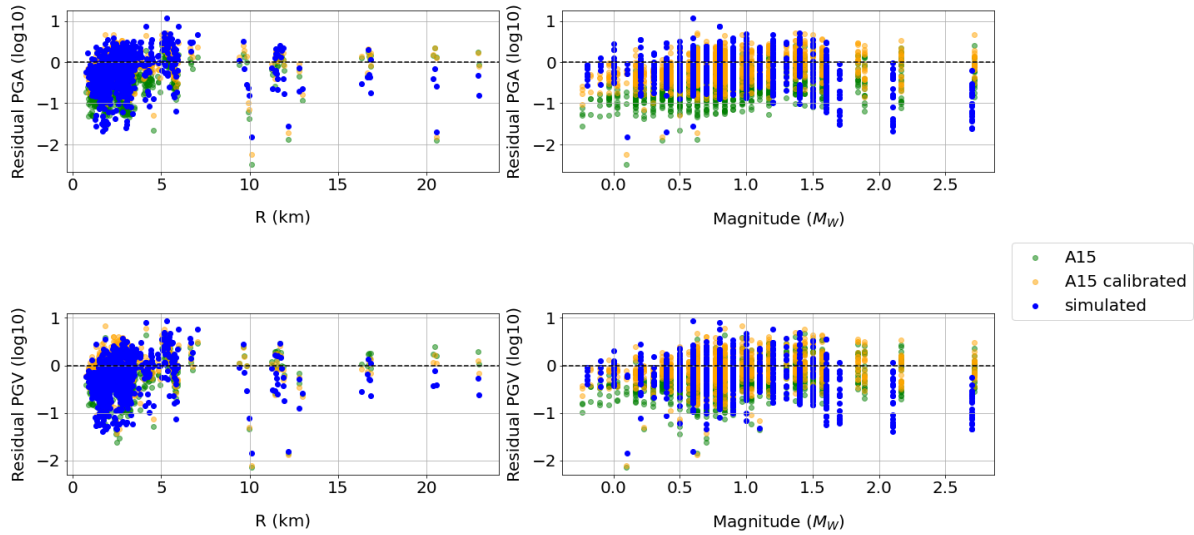
## 375 General Discussion and Conclusions

376 Better determination of seismic hazard is one of the important factors for a better risk  
377 assessment. Seismic hazard assessment by adapting and adjusting the existing GMPE for near  
378 field- shallow earthquakes has been evaluated in this study specifically for PNR site. In this work,  
379 the attenuation parameter known as the quality factor ( $Q$ ) and site-specific high-frequency decay  
380 ( $\kappa_0$ ) of UK tectonic and PNR induced seismicity datasets from previous study by Suroyo &  
381 Edwards (2023) were incorporated to develop a new physical-based ground motion model.

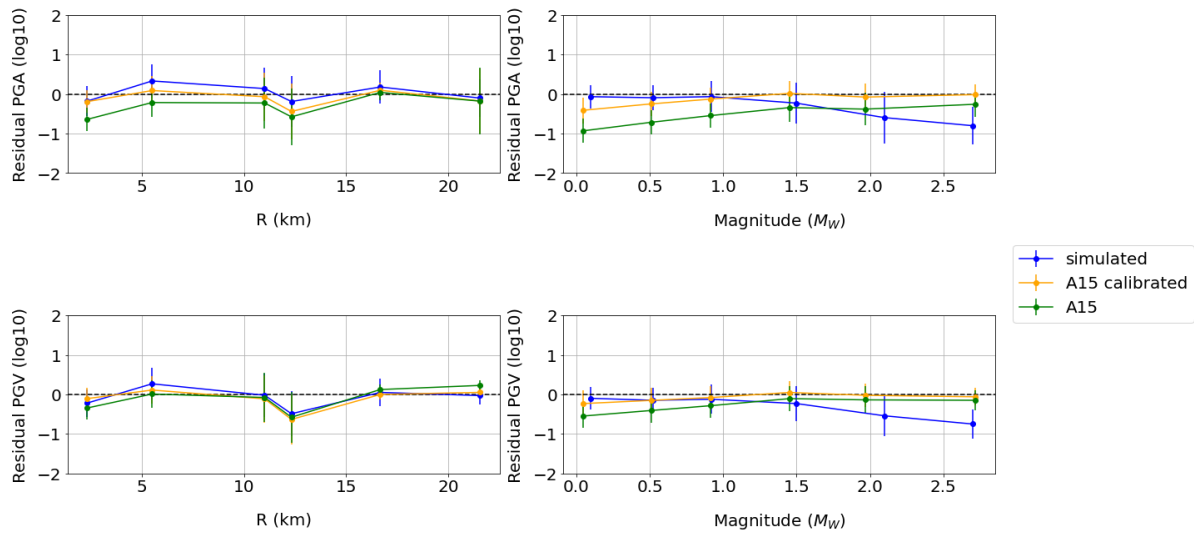
382 The Fourier spectrum of the ground motion is essential information for developing a physical-  
383 based ground motion model, which reflect the contributions of the earthquake source, wave  
384 propagation, and site amplification. A preliminary study to better understand source, path, and site  
385 characteristics for induced seismicity at the PNR site and UK tectonic seismicity has been  
386 conducted by Suroyo & Edwards (2023) using spectral fitting approach. For the path term,  
387 attenuation models obtained are  $Q_T = 678.3f^{0.4}$  and  $Q_I = 108.2f^{0.15}$  for tectonic and induced  
388 seismicity, respectively. Average site-specific high-frequency decay ( $\kappa_0$ ) noted as 0.029 s for  
389 tectonic and 0.018 s for induced dataset (Suroyo & Edwards, 2023). These model parameters are  
390 then used to perform the stochastic simulation. This report presented the comparison between  
391 simulated PGA and PGV with empirical prediction following GMPEs from Atkinson (2015), and  
392 a calibrated version of Atkinson's GMPE (in this study known as the Edwards et al. (2021) model).

393 The difference between the predicted value and the observation (residual value) is presented in  
394 Figures 10 and 12. Figures 11 and 13 show additional illustrations of the binned average and its  
395 standard deviation to represent the mean residual value for a given bin and the variability within  
396 the bin. The results indicate that the existing GMPE for induced seismicity developed by Atkinson  
397 (2015) underpredicts the ground motion at the PNR site, while the calibrated GMPE model by  
398 Edwards et al. (2021) shows improved prediction, despite the uncertainty that follows the  
399 prediction. This emphasizes the importance of local calibration or local-specific in predicting  
400 ground motion. Residuals of the simulated PGA and PGV for induced dataset show a relatively  
401 good fit at lower magnitudes and slightly underestimate the observation at magnitudes  $> 1.5$   
402 (Figures 10 and 11). This might explain the necessity of using the scaling of stress drop with  
403 magnitude in the simulation.



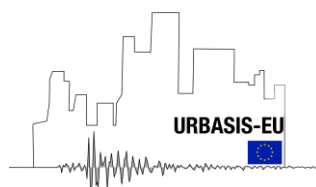


404  
 405 *Figure 10. Residuals [in log-10] for PGV (below) and PGA (upper) versus magnitude (right) and distance (left) obtained from*  
 406 *PNR- induced dataset (2018-2019). Green dots refer to the calculated PGV and PGA using Atkinson (2015) model, while yellow*  
 407 *dots correspond to calibrated model (Edwards et al., 2021) and blue dots are the simulated PGV and PGA.*

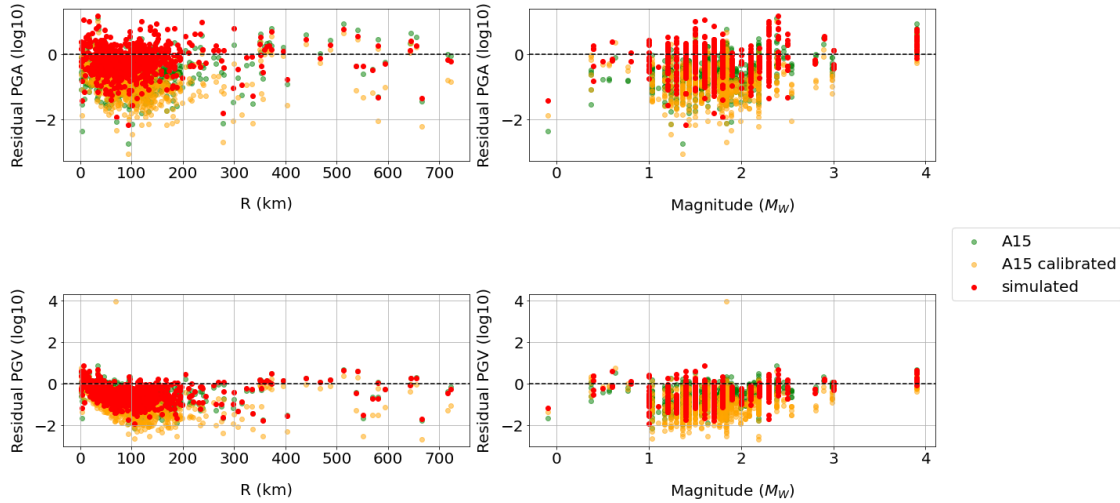


408  
 409 *Figure 11. Binned average of residual values [in log-10] for PGV (below) and PGA (upper) versus magnitude (right) and distance*  
 410 *(left) obtained from PNR- induced dataset (2018-2019). Green dots refer to the calculated PGV and PGA using Atkinson (2015)*  
 411 *model, while yellow dots correspond to calibrated model (Edwards et al., 2021) and blue dots are the simulated PGV and PGA.*  
 412 *Number of bins = 6, with interval width equal to 3.77 km for residual vs distance (left) and 0.5 Mw for residual vs magnitude (right).*

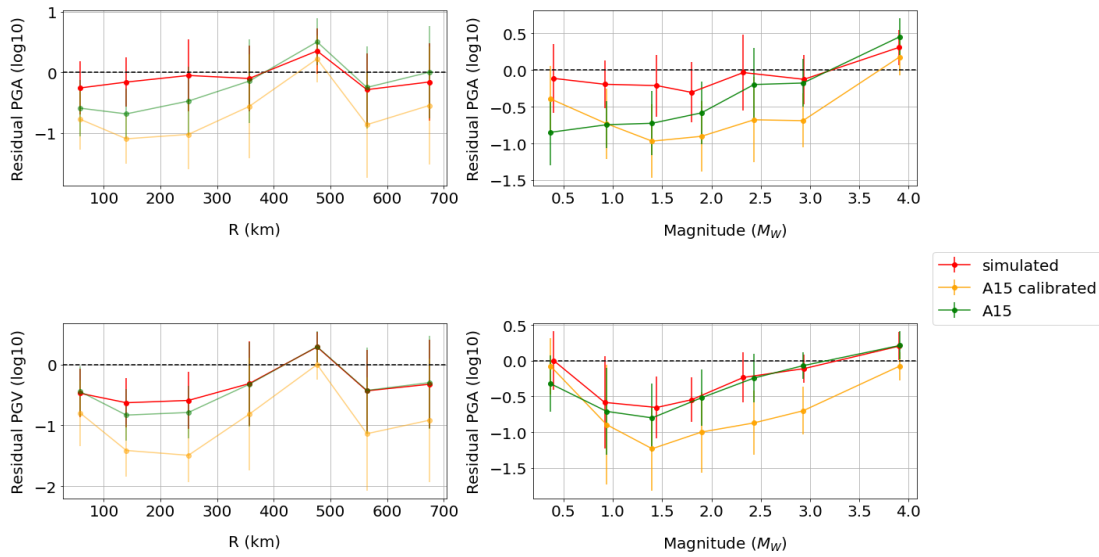
413





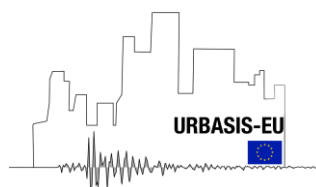


414  
 415 *Figure 12. Residuals [in log-10] for PGV (below) and PGA (upper) versus magnitude (right) and distance (left) obtained from*  
 416 *UK tectonic dataset (2019). Green dots refer to the calculated PGV and PGA using Atkinson (2015) model, while yellow dots*  
 417 *correspond to calibrated (Edwards et al., 2021) and red dots are the simulated PGV and PGA.*



418  
 419 *Figure 13. Binned average of residual values [in log-10] for PGV (below) and PGA (upper) versus magnitude (right) and distance*  
 420 *(left) obtained from UK tectonic dataset (2019). Green dots refer to the calculated PGV and PGA using Atkinson (2015) model,*  
 421 *while yellow dots correspond to calibrated (Edwards et al., 2021) and red dots are the simulated PGV and PGA. Number of bins*  
 422 *=7, with interval width equal to 102.87 km for residual vs distance (left) and 0.6  $M_w$  for residual vs magnitude (right).*

423 Higher variability is observed in the residual results for tectonic dataset (Figures 12 & 13). It is  
 424 noted that the empirical prediction produces slightly lower residual value compared with simulated  
 425 prediction. Overall, the residual plot of the tectonic dataset shows underprediction for both  
 426 empirical (Atkinson's model and calibrated model) and simulated prediction. Examples of PSA

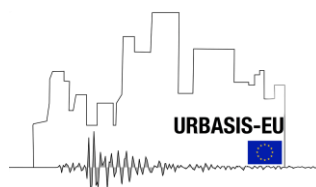


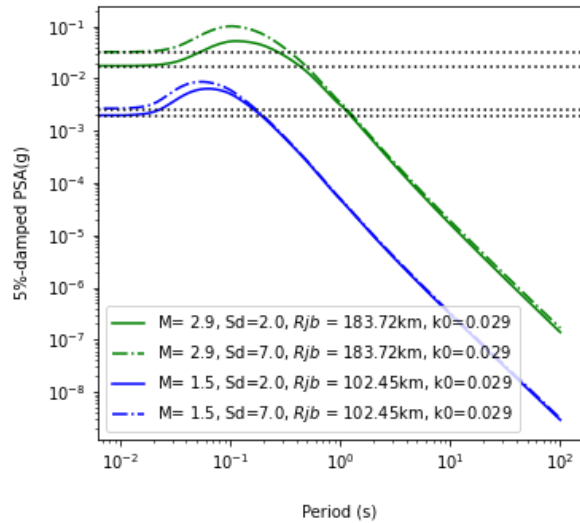
427 with respect to period for two different tectonic events: 2.9  $M_w$  recorded at 183.72 km and 1.5  $M_w$   
428 at 102.4 km distances with two different stress drop inputs are given in Figure 14.

429 The comparison of model of Atkinson (2015), calibrated model by Edwards et al. (2021),  
430 simulated PGV, and observed PGV for two events (2.9 and 1.5  $M_w$ ) is illustrated in the Figure 15,  
431 in addition to a similar comparison of two examples of induced seismic events (0.4 and 1.6  $M_w$ ).  
432 The comparison for induced seismicity case (Figure 15a) reveals that the empirical GMPEs (A15,  
433 and calibrated) are slightly overestimate the observation at  $R > 5$  km for 1.5  $M_w$  event, while the  
434 simulated PGV appears to give promising results. However, the simulated PGV shows that the  
435 rapid decay of near-field motions is confined to the majority of small ( $M_L < 1.5$ ) events which is  
436 shown by the underprediction at a near distance ( $R < 5$  km) for 1.6  $M_w$  earthquake, although the  
437 simulated PGV is comparable to the observed PGV for smaller events. As for the tectonic  
438 earthquake case shown in Figure 15.b, the empirical and stochastic prediction tend to underpredict,  
439 particularly at distance greater than 40 km. The underprediction founds to be higher at a lower  
440 magnitude. This can be due to the selection of the GMPE model, which is better suited for the  
441 induced seismic case. The GMPE models used in this study were developed and calibrated for  
442 near-source distance earthquakes ( $R < 40$  km), with near-source distance saturation considered. In  
443 fact, for tectonic earthquakes, as the distance increases, the near-source distance saturation should  
444 be disregarded. Although the simulated PGV predicts better than the empirical PGV, the bias of  
445 the prediction is still large, and the simulation needs to be optimised.

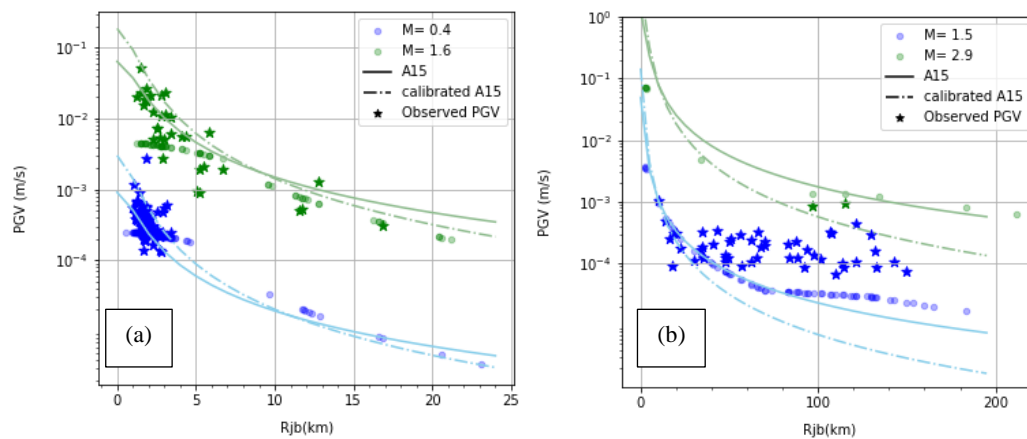
446 Finally, this work demonstrates the different characteristics of ground motion model for near  
447 field- shallow earthquake and the deeper tectonic earthquake. Selecting proper and suitable ground  
448 motion model is a crucial part for hazard assessment. Our result illustrates the use of near-source  
449 distance saturation for large magnitude (tectonic) events may result in bias, and directly adapting  
450 tectonic GMPEs for induced seismicity is also tricky and not a suitable solution. We suggest  
451 implementing the rapid decay of near-field motions to lower magnitude ( $M_L < 1.5$ ) and short  
452 distance events. Further analysis about the uncertainty and variability of the input model parameter  
453 to minimise the misfit and better predict the ground motion is needed.

454





455  
 456 Figure 14. Response spectra computed with time-domain simulations for 2.9 and 1.5  $M_w$  (tectonic events). Black dashed lines show  
 457 the corresponding PGA values.



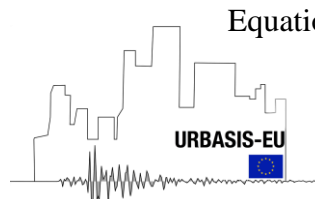
458  
 459 Figure 15. Observed PGV for: (a) induced dataset with magnitude 0.4  $M_w$  and 1.6  $M_w$  and (b) tectonic dataset with magnitude 1.5  
 460  $M_w$  and 2.9  $M_w$ , compared to the model of Atkinson (2015), calibrated model by Edwards et al. (2021), and simulated PGV. Circles  
 461 are simulated values.

462

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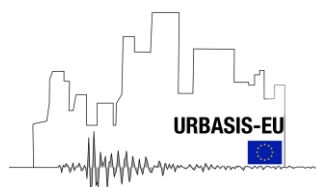
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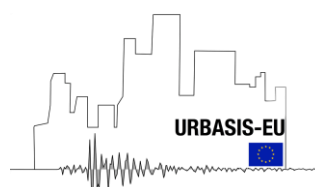
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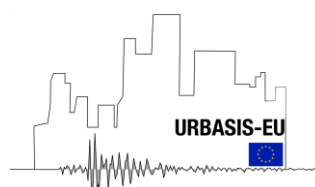
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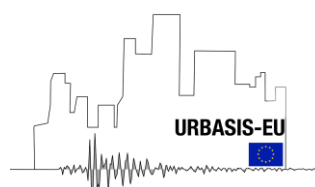
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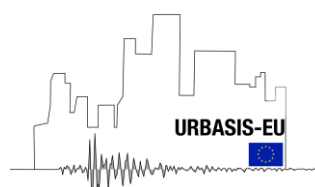
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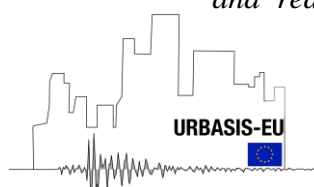
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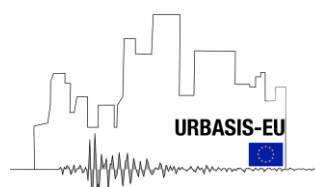
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